

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\theta = \frac{1}{2} \arctan \left(\frac{2\tau_{xy}}{\sigma_x - \sigma_y} \right)$$

$$I_{xx} = \int y^2 dA, I_{yy} = \int x^2 dA, I_{xy} = \int xy dA, I_{xx'} = I_{xx} \cos^2 2\theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 2\theta$$

3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x \sigma_y + \sigma_x \sigma_z + \sigma_y \sigma_z$$

$$I_3 = \sigma_x \sigma_y \sigma_z - \tau_{xy} \tau_{yz} - \tau_{yz} \tau_{xz} - \tau_{xz} \tau_{xy}$$

$$I_4 = \sigma_x \tau_{xy} \tau_{yz} + \sigma_y \tau_{yz} \tau_{xz} + \sigma_z \tau_{xz} \tau_{xy} - \tau_{xy}^2 \sigma_z - \tau_{yz}^2 \sigma_x - \tau_{xz}^2 \sigma_y$$

$\sigma_{1,2,3}$ = roots of $\lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$

$$\tau_{max} = \frac{\sigma_1 - \sigma_3}{2}$$

See ex. for principal direction

$$I_{xx'} = I_{xx} \cos^2 \theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 \theta$$

$$I_{yy'} = I_{yy} \cos^2 \theta - I_{xy} \sin 2\theta + I_{xx} \sin^2 \theta$$

$$I_{xy'} = I_{xy} \cos 2\theta + \frac{I_{xx} - I_{yy}}{2} \sin 2\theta$$

Centroidal Bending

$$\sigma_{max} = \sum \frac{M y}{I} + \frac{P}{A}$$

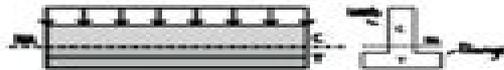
C is distance from NB to point of interest; it differs for circular shaft

I & A will be the same

bending stress will be the same (signified) side

POA is used (signified) stress distribution

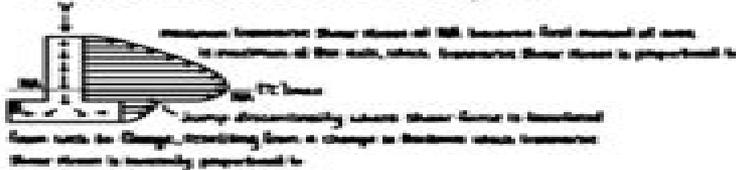
tensile bending → positive, compression bending → negative



Transverse Shear

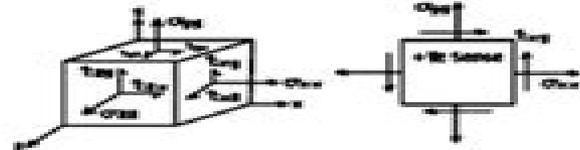
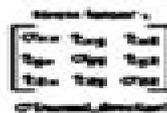
$$\tau = \frac{VQ}{It}$$

Q → Area, for rectangles along axis but I & Q are with respect to I & Q will be the same opposite to V (another axis (signified))



Torsion

$$\tau = \frac{T\rho}{J}$$



Plane Sectional Properties

$$Centroid: \bar{y} = \frac{\sum y_i A_i}{\sum A_i}$$

Second Moment of Area

$$I = \sum I_{xxi} + A_i \bar{y}_i^2$$

$$I_y = \sum I_{yyi} + A_i \bar{x}_i^2$$

if I_{xy} , have that in face perspective of y -axis

if I_{xy} , " " x -axis

Area Moment of Inertia

$$J = \sum (I_{xxi} + I_{yyi})$$

$I_x = \sum I_{xxi}$ if bar is used as a shaft; hollow one $\sigma_c = \frac{M}{I}$

Equal Moment of Area

moment of area about or below point of interest

$$Q = A\bar{y}$$

where A is the remaining cross sectional area about or below that point

and \bar{y} is the distance between the centroid of that area to the centroid of the entire cross section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small

like a plate or a shell, stress with large constant

(2) — total has to be in the plane of the plate, etc., same way it does not.

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading condition

Stress-Strain

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix}$$

Stress-Strain Relationship

$$E_{11} = E(\nu^2 + 1) / (1 - \nu^2)$$

$$E_{22} = E(\nu^2 + 1) / (1 - \nu^2)$$

$$E_{33} = E(\nu^2 + 1) / (1 - \nu^2)$$

$$E_{12} = E\nu / (1 - \nu^2), E_{13} = E\nu / (1 - \nu^2), E_{23} = E\nu / (1 - \nu^2)$$

$$G = E / (2(1 + \nu))$$

Plane Stress Condition

$$E_{11} = E / (1 - \nu^2)$$

$$E_{22} = E / (1 - \nu^2)$$

$$E_{33} = -\nu E / (1 - \nu^2)$$

$$E_{12} = E\nu / (1 - \nu^2)$$

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