

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\left(\frac{\sigma_x - \sigma_y}{2}\right) \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\theta = \frac{1}{2} \arctan \left(\frac{2\tau_{xy}}{\sigma_x - \sigma_y} \right)$$

$$I_{xx} = \int y^2 dA, I_{yy} = \int x^2 dA, I_{xy} = \int xy dA, I_{xx'} = I_{xx} \cos^2 2\theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 2\theta$$

3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x \sigma_y + \sigma_x \sigma_z + \sigma_y \sigma_z$$

$$I_3 = \sigma_x \sigma_y \sigma_z - \tau_{xy} \tau_{yz} - \tau_{yz} \tau_{zx} - \tau_{zx} \tau_{xy}$$

$$I_4 = \sigma_x \sigma_y \sigma_z (\sigma_x + \sigma_y + \sigma_z) - \tau_{xy}^2 (\sigma_x + \sigma_z) - \tau_{yz}^2 (\sigma_y + \sigma_x) - \tau_{zx}^2 (\sigma_y + \sigma_z)$$

$\sigma_{1,2,3}$ = roots of $\lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$

$$\tau_{max} = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}{2}}$$

See ex. for principal direction

$$\tau_{max} = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}{2}}$$

$$\tau_{max} = \sqrt{\frac{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}{2}}$$

Centroidal Bending

$$\sigma_{max} = \frac{M y}{I}$$

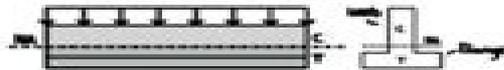
C is distance from NB to point of interest; y is taken for counter-clockwise

I is NB with the MCT neutral axis

bending stress will be MCT (negative) side

MCA is neutral (deformed) stress distribution

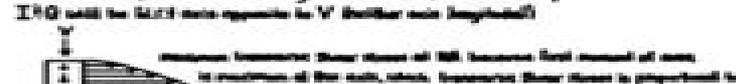
tensile bending → positive, compression bending → negative



Transverse Shear

$$\tau = \frac{VQ}{It}$$

Q is $\int y dA$, for rectangles along axis but I & Q are with respect to I & Q with the MCT axis opposite to V (other axis (horizontal))



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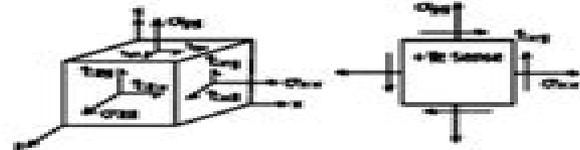
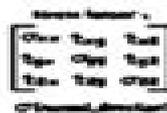
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Case Sectional Properties

$$C_{centroid} = \frac{\sum (y_i A_i)}{\sum A_i}$$

Second Moment of Area

$$I = \sum (I_{i,centroid} + A_i d_i^2)$$

$$d_i = (y_i - y_c)$$

If I_{yy} , take d_i in face perspective of y -axis

If I_{xx} , " " x -axis

Area Moment of Inertia

$$J = \sum (I_{i,centroid}^2 + A_i d_i^2)$$

$I_x = \int y^2 dA$ if bar is used as a beam

Equal Moment of Area

moment of area about or below point of interest

$$Q = A y_c$$

where A is the remaining case sectional area about or below that point

and y_c is the distance between the centroid of that area to the centroid of the entire case section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small

(2) — like a plate or a shell, stress with large constant

(3) — total has to be in the plane of the plate, etc., same way it does not

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading condition

Stress-Strain

A' : point of interest

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & \nu_{12} & \nu_{13} \\ \nu_{21} & E_{22} & \nu_{23} \\ \nu_{31} & \nu_{32} & E_{33} \end{bmatrix}$$

Stress-Strain Relationship

$$E_{11} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33} - \nu_{12} \nu_{13} E_{33}$$

$$E_{22} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33} - \nu_{12} \nu_{13} E_{33}$$

$$E_{33} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33} - \nu_{12} \nu_{13} E_{33}$$

$$E_{12} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33} + \nu_{12} \nu_{13} E_{33}$$

$$C = G(1 + \nu)$$

Plane Stress Condition

$$E_{11} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{22} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{33} = -\nu_{12} \nu_{13} E_{33} - \nu_{12} \nu_{13} E_{33}$$

$$E_{12} = E \nu_{12} \nu_{13}$$

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