

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

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3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x\sigma_y + \sigma_y\sigma_z + \sigma_z\sigma_x - \tau_{xy}^2 - \tau_{yz}^2 - \tau_{zx}^2$$

$$I_3 = \sigma_x\sigma_y\sigma_z + 2\tau_{xy}\tau_{yz}\tau_{zx} - \sigma_x\tau_{yz}^2 - \sigma_y\tau_{zx}^2 - \sigma_z\tau_{xy}^2 + 3\det(\text{stress})$$

Principle stress of 1, 2, 3, where

Principle stress with maximum principal stress

Principle stress with minimum principal stress

Principle stress with intermediate principal stress

Centroidal Bending

$$\sigma_{max} = \sum \frac{M y}{I} + \frac{P}{A}$$

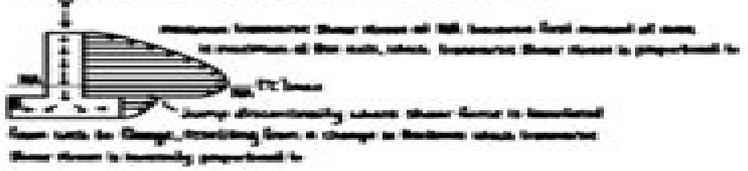
C is distance from NB to point of interest; it differs for circular shaft
 I is NB with the MCT neutral axis
 bending stress will be MCT longitudinal axis
 P/A is axial (longitudinal) stress contribution
 tensile bending → positive, compression bending → negative



Transverse Shear

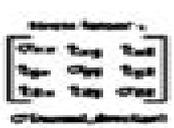
$$\tau = \frac{VQ}{It}$$

Q → Area, for rectangles along axis but I/Q are with respect to I/Q will be MCT axis opposite to V (another axis longitudinal)



Torsion

$$\tau = \frac{T\rho}{J}$$



Plane Sectional Properties

Centroid: $\bar{y} = (\sum y_i A_i) / (\sum A_i)$

Second Moment of Area: $I = \sum (I_{centroidal} + A_i d_i^2)$ only use if other axis is not coincident

$d_i = (y_i - \bar{y})$

If I_{yy} , lower (b) is face perspective of y -axis

If I_{xx} , " " x -axis

Area Moment of Inertia: $J = \int r^2 dA$ if bar is circular or shaft; hollow use $d_o^4 - d_i^4$

$I_x = \int y^2 dA$ if bar is used as a beam

First Moment of Area: moment of area above or below point of interest

$Q = A\bar{y}$

where A is the remaining cross sectional area above or below that point and \bar{y} is the distance between the centroid of that area to the centroid of the entire cross section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small like a plate or a shell. Stress with large constant

(2) — shell has to be in the plane of the plate, etc., same way it does not.

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading condition

Stress-Strain

A: point of interest

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & \nu_{12} & \nu_{13} \\ \nu_{21} & E_{22} & \nu_{23} \\ \nu_{31} & \nu_{32} & E_{33} \end{bmatrix}$$

symmetry

elasticity relation

Stress-Strain Relationship

$$E_{xx} = E(\nu_{xy}\nu_{yx} - \nu_{xy}\nu_{yz} - \nu_{yz}\nu_{xy})$$

$$E_{yy} = E(\nu_{xy}\nu_{yx} - \nu_{xy}\nu_{xz} - \nu_{xz}\nu_{xy})$$

$$E_{zz} = E(\nu_{xy}\nu_{yx} - \nu_{xy}\nu_{xz} - \nu_{xz}\nu_{xy})$$

$$E_{xy} = E(\nu_{xy}\nu_{yx} + \nu_{yz}\nu_{xz} + \nu_{xz}\nu_{yz})$$

$$G = E(2(1+\nu))$$

Plane Stress Condition

$$E_{xx} = E(\nu_{xy}\nu_{yx} - \nu_{xy}\nu_{yz})$$

$$E_{yy} = E(\nu_{xy}\nu_{yx} - \nu_{xy}\nu_{xz})$$

$$E_{zz} = -\nu_{yz}\nu_{xy} - \nu_{xz}\nu_{xy}$$

$$E_{xy} = E\nu_{xy}\nu_{yx}$$

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